

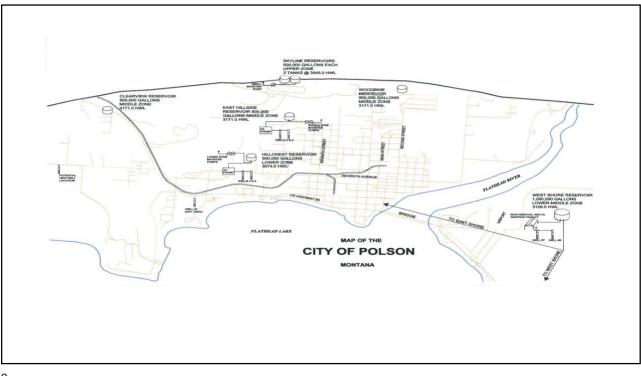
ensus Date	Population	Annual Average	
		Growth Rate	
1960	2,314	-	
1970	2,464	0.65%	
1980	2,798	1.36%	
1990	3,291	1.76%	
2000	4,041	2.28%	
2010	4,488	1.11%	
2019 ¹	5,060	1.42%	
	1980 1990 2000 2010 2019 ¹	19802,79819903,29120004,04120104,488201915,060	19802,7981.36%19903,2911.76%20004,0412.28%20104,4881.11%

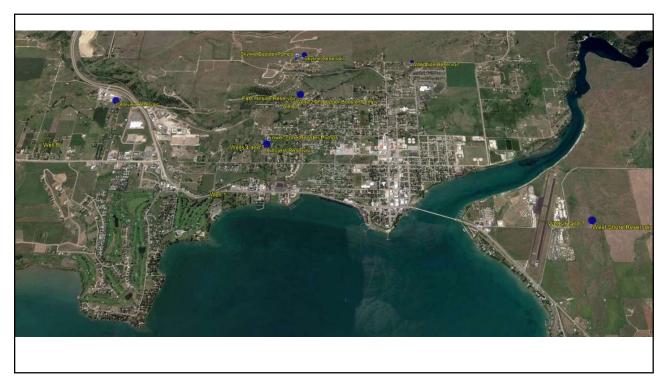
	Table 1-5. Population F	Projections at 0.65%, 1.3	7%, and 2.28%	
Year	Actual Census Population	Population (Projections at 0.65% per year)	Population (Projections at 1.37% per year)	Population (Projections at 2.28% per year)
1990	3,291			
2000	4,041			
2010	4,488			
2019	5,060			
2021		5,126	5,200	5,293
2026		5,295	5,566	5,925
2031		5,469	5,957	6,632
2036		5,649	6,377	7,423
2041		5,835	6,826	8,309
0.65% = His	storical 50-year growth rate. 1.37	% = Growth rate in 2016 Growth	Policy. 2.28% = Highest growt	h rate in last 50 years.

Year	Annual Metered Water (million gallons)	Population Estimate for 2019 ¹	Gallons Per Capita Per Day (GPCD)
2018	245		
2019	229		
2020	232		
Average Day Demand	644,749 GPD	5,060	127
It should be noted that commercial, industrial,	9 is the estimate from census data. t the gallons per capita per day repr and irrigation), divided by the total ne, GPCD should not be used as a	population. Given the wid	le variation in customer

Table 1-11. Projected Pop	oulation and Water	Demands for 2041	
Growth Rates	0.65% per year	1.37% per year	2.28% per year
Population	5835	6826	8309
Average Day (Million Gallons per Day)	0.743	0.870	1.06
Average Day (Gallons Per Minute)	516	604	735
Maximum Day (Million Gallons per Day)	2.23	2.61	3.18
Maximum Day (Gallons Per Minute)	1,549	1,811	2,205
Peak Hourly Flow (Million Gallons Per Day)	5.58	6.53	7.95
Peak Hourly Demand (Gallons per Minute)	3,873	4,528	5,513
Average Year (Million Gallons)	271	317	386
Acre-feet per Year	833	974	1,186

For planning, modeling and design purposes, the data associated with the 1.37% annual population growth will be used moving forward.





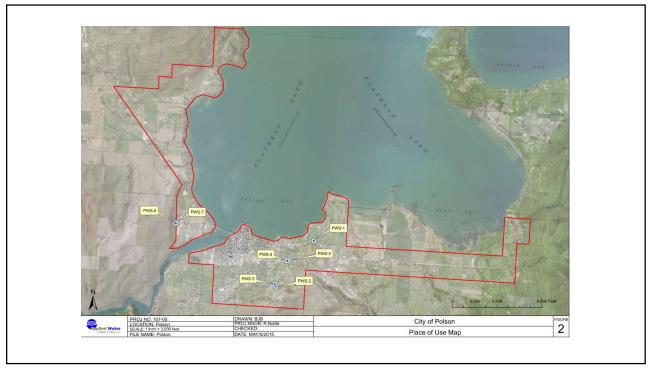


	Table 2-1. V	Vell Pumping Capacities	
Well Name	Recorded Pumping Rate (gpm)	Design Rated Capacity (gpm)	Reservoir
Well 1		450	N/A
Well 2	377	500	Hillside
Well 3	508	520	Hillside
Well 4	220	200	Devlin/ Hillcrest
Well 5		50	Devlin/ Hillcrest
Well 6	450	425	West Shore
Well 7		425	West Shore
Well 8 ¹	625	625	Will Tie-in to existing main

Additionally, the City has Surface Water Rights for Hell Roaring Creek Surface Water Supply

Table 2-2. Pressure Zones				
Pressure Zone	Upper Elevation	Lower Elevation		
Lower	2,902	3,073		
Lower-Middle	2,890	2,966		
Middle	2,927	3,162		
Upper	3,044	3,418		

Tal	ble 2-4. Pipe Ma	terial by Length		_	Table 2-	5. City of Polson Water S	torage Reser	voirs
Material	Total Length (ft)	Total Length (miles)	Percent of System	v	Vater System Reservoirs	s Reservoir Capacity (Gallons)	Туре	Zone
PVC	234,913	44.49	79.07%					
Cast Iron	46,385	8.79	15.61%		Hillcrest	500,00	Concrete	Lower Zone
Ductile Iron	819	0.16	0.28%		West Shore	1,000,000	Concrete	Lower-Middle Zone
Asbestos Cement	317	0.06	0.11%		Clearview	500,000	Concrete	Middle Zone
Galvanized Iron	12,725	2.41	4.28%		East Hillside	500,000	Steel	Middle Zone
Steel	214	0.04	0.07%		Woodbine	500,000	Concrete	Middle Zone
Galvanized	1,715	0.32	0.58%					
Total	297,086	56.27	100.00%		Skyline 1	500,000	Steel	Upper Zone
					Skyline 2	500,000	Concrete	Upper Zone
			Tabl	e 2-6. B	ooster Stations			
		Booster I	Pump Stati	on	Zone	Total Capacity (MGD)		

Booster Pump Station	Zone	Total Capacity (MGD)
Devlin/ Hillcrest Booster Pumps	Lower Zone	940 gpm
East Hillside Booster Pumps	Middle Zone	600 gpm
Skyline Booster Pumps	Upper Zone	1783 gpm

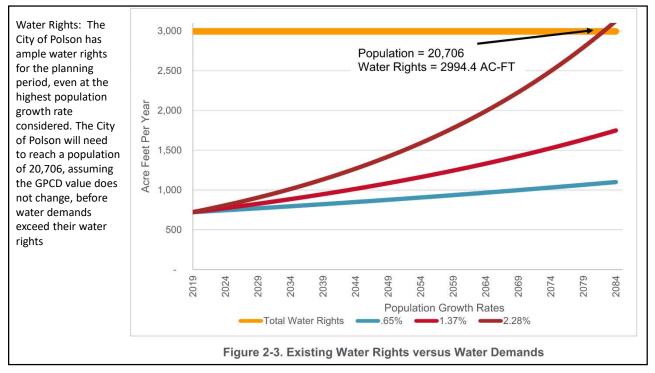


Table 2-10. Co	ondition Assessment Summary
Facility	Items Noted
Water Storage	
East Hillside	Built in 1960, steel. The East Hillside Steel Reservoir is in very poor condition. According to City staff, the internal support beams are heavily corroded, and the shell of the reservoir is significantly warped beyond repair. The City would prefer to demolish this steel reservoir and replace with a concrete inground alternative at the same site location.
Booster Stations	
Devlin/ Hillcrest Booster Pumps (Lower)	The Hillcrest and East Hillside booster pump stations are both aged and replacement of major equipment should be completed soon as replacement parts are located (they have been difficult to find). Pump house and piping are in good condition. Electrical upgrades, SCADA, and pump replacement are necessary.
East Hillside Booster Pumps (Upper)	The Hillcrest and East Hillside booster pump stations are both aged and replacement of major equipment should be completed soon as replacement parts are located (they have been difficult to find). Electrical upgrades, SCADA, and pump replacement are necessary.
	Continued next slide

Wells	
#1	Built in 1969. Original capacity was 425 gpm, but in 1999 it started pumping fine material and capacity was reduced to 100 gpm. The pump sand pack collapsed due to over pumping during installation of a well pump and attempts to renovate the well have been unsuccessful.
#5	Built in 2000. Treated with polyphosphate corrosion inhibitor and sodium hypochlorite for disinfection. Staff indicates that this well pumps sand, therefore, it has not been used in almost 20 years. Well 5 is currently not running due to sand issues that will be resolved with investigations including an air burst test and potentially redeveloping the well or a sand separator.
#7	Built in 2011. Includes an iron removal treatment system. No emergency power. Well #7 is not currently running. The electrical controls for the iron removal filter are not operating properly and need to be reprogrammed.
	Continued next slide

<u>PRVs</u>	
	Demers needs to be replaced. The PRV sags really bad due to no supports being used at initial installation.
	Off of Claffey (maybe called 'hillside'? PRV) from 1970's and never been opened or touched, never been repaired. "Oldest valves ever seen"
	1st street and 12th ave west, a PRV is in poor condition
	PRV across from 4-B's on Hwy 35 and poor condition and 25 feet below ground.
<u>Controls</u>	
	New SCADA system is necessary. Forthcoming replacement of entire system.

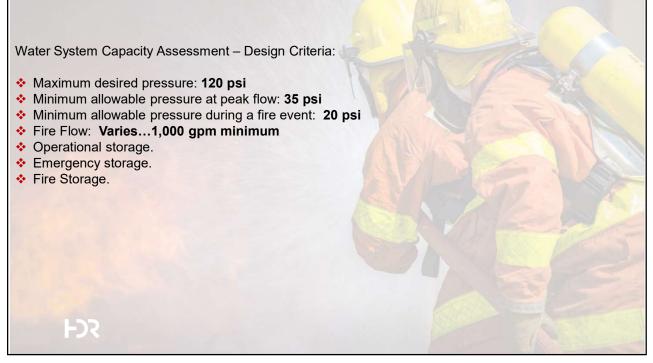
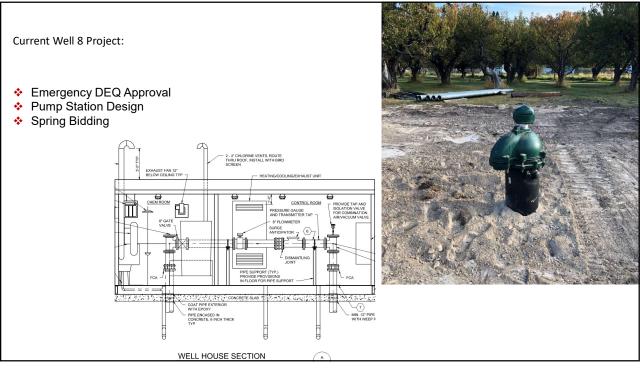


		Table 3-8. Locat	ions of Fire Flow D	eficiency
Pressure Zone	Figure Number	General location	Available Fire Flow (gpm) at 20 psi	Needs/Recommendation
	3-19	Bayview Drive/Polson Bay Golf Course	667.5-960.7	 Additional supply, new location in area for Well 1 and bringing back in service.
	3-19	Polson Bay Golf Course	115.7	Increase 2" main at golf course
	3-20	1 st St E and 2 nd St E	788.3-926.7	 Increase 2" main on 1st St. E to 8", and connect 6" main from 1st Ave E to US Hwy. 93.
Lower Zone	3-21	8 th St E and 5 th Ave E	14.16-916.7	 Construct 6" main on 8th St E from 5th Ave E to US Hwy 93, and connect existing hydrant at intersection of US Hwy 93 and 8th St E.
Lower Zone	3-21	Lindeman School vicinity	717.1-831.9	 Connect 6" main along 4th Ave E, and connect 6" main along 6 Ave E.
	3-22	6 th Ave W and 4 th St W	176.3	Increase 2" main along 4 th St W to 6".
	3-22	Polson Sports Complex Vicinity	881.8-985.8	 Construct 8" main down Kerr Dam Rd to connect loop.
	3-22	11 th Ave W and 3 rd St W	901.1	• Connect 6" main along 11 th Ave W.
	3-23	Division St and 1 st St E	112.9/420.5	Reconnect hydrants to 6" main along 2 nd St E
	3-24	16 th Ave E and 8 th St E	819.2	Construct 8" main down 8th St E.
Middle Zees	3-25	Providence St. Joseph Hospital Vicinity	637.5-997.7	 Connect 6" main and 12" main on 2nd St W in front of Polson High School.
Middle Zone	3-26	Southlake Crest	951.7-979.6	 Connect 8" main on Skyview Lane to 8" main on Mission View Dr.
	3-27	Lavista Dr.	116.9	 Increase 3" main along Lavista Dr to 6".
	3-19	Montana Landing	972.11-999.9	Connect 8" main on Skyview Lane to 8" main on Mission View Dr.
Upper Zone	3-28	Mission View Dr	988.6-997.8	Connect 8" main on Skyview Lane to 8" main on Mission View Dr.
Middle/Skyline	3-29	Hillside Tank Vicinity	750.6/979.8	Increase PRV 9 setting to 25 psi.

- 1				And the second second second	2 Hour Deman	d (apm)	
	Zone	Sup	pply Design Point (gpm)	Storage (MG)	2041 Proje		
	Lower Middle		Well 6 – 600 gpm Well 7 – Off	Westshore – 1.03 MG	155 (max day) + 1,5	500 (fire flow)	
	Middle	Hill	Well 2 – 1,000 gpm Well 3 - Off Well 8 – 625 gpm crest PS – 800 gpm Full	Woodbine – 0.49 MG Clearview – 0.5 MG Hillside – 0.53 MG	883 (max day) + 1,8	500 (fire flow)	
	Lower		Well 4 – 300 gpm Well 5 - Off	Hillcrest – 0.54 MG	802 (max day) + 1,8	500 (fire flow)	
	Greater Upper (Skylir Upper, End, Intermedia		lside PS – 310 gpm Full	Skyline 1 – 0.5 MG Skyline 2 – 0.5 MG	427 (max day) + 1,8	500 (fire flow)	
	Table 3-11. Stora	age and I	Pumping Capacity Ar	nalysis – Two-Hour S	torage Deficit/Sur	plus (2041)	
	Zone		orage Needed um Day Demands	Storage N Maximum Day Dema		Half-Full S (gallor	
L	ower Middle	Sources	> Demands	127,000 gallor	is needed	515,00	00
	Middle	Sources	> Demands	Sources > D	emands	761,00	00
	Lower	60,000 (Gallons Needed	240,000 gallor	is needed	268,00	00
	er Upper (Skyline, End, Intermediate)	70,000 (Gallons Needed	194,000 gallor	ns needed	503,00	00

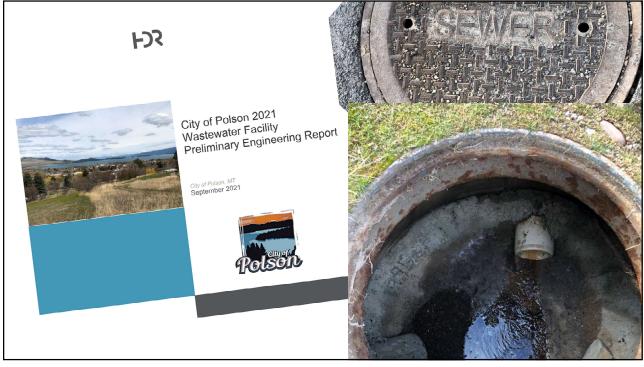


	Та	ible 3-13. Sum	mary of Needs	
Need	Category	Project Type	Recommended Timeline	Consequence if not Corrected
Construct new Well 1	Fire Flow Deficiency	Supply Well	Near Term -1 to 3 years	Fire flow deficiencies in northern section of Lower Zone.
East Hillside Reservoir Replacement and SCADA	Aging Infrastructure	Water Storage	Near Term -1 to 3 years	The tank will continue to deteriorate and eventually fail.
Hillcrest Booster Rehabilitation	Aging Infrastructure	Booster Station	Near Term -1 to 3 years	Aging pumps are nearing their point of failure and parts are impossible to replace due to age. Building is in good condition. Project include new pumps, and SCADA and electrical upgrades.
Increase 3" main along Lavista Dr to 6".	Fire Flow Deficiency	Water main	Mid Term – 3 to 5 years	No fire flow to Lavista Drive.
Connect 8" main on Skyview Lane to 8" main on Mission View Dr.	Fire Flow Deficiency	Water main	Mid Term – 3 to 5 years	Fire flow deficiencies on Mission View Drive and Southlake Crest.
Construct 6" main on 8 th St E from 5 th Ave E to US Hwy 93, and connect existing hydrant at intersection of US Hwy 93 and 8 th St E.	Fire Flow Deficiency	Water main	Mid Term – 3 to 5 years	Fire flow deficiencies on 8 th Street East in heavily residential area.
Connect 6" main along $4^{\mbox{th}}$ Ave E from 5th St E west to the main in front of the Linderman School.	Fire Flow Deficiency	Water main	Near Term -1 to 3 years	Fire flow deficiencies near Linderman School.
Tie in the 6" main along 6 Ave E.				
Connect 6" main and 12" main on 2 nd St W in front of Polson High School.	Fire Flow Deficiency	Water main	Mid Term – 3 to 5 years	Fire flow deficiencies near Polson High School.
Increase PRV 9 setting to 25 psi.	Fire Flow Deficiency	PRV	Near Term -1 to 3 years	Fire flow deficiencies on 15th Ave East.
Additional hydrants in 11th Street E Vicinity	Fire Flow Deficiency	Hydrant Installations	Mid Term – 3 to 5 years	No fire flow coverage in area, additional hydrants required.
Additional hydrants in 12 th Avenue E Vicinity	Fire Flow Deficiency	Hydrant Installations	Mid Term – 3 to 5 years	No fire flow coverage in area, additional hydrants required.
Additional hydrants along South Shore Route	Fire Flow Deficiency	Hydrant Installations	Mid Term – 3 to 5 years	No fire flow coverage in area, additional hydrants required.
Water System SCADA Improvements/Upgrades	Aging Infrastructure	SCADA	Near Term -1 to 3 years	SCADA system is out of date and in need of upgrades and additional functionality

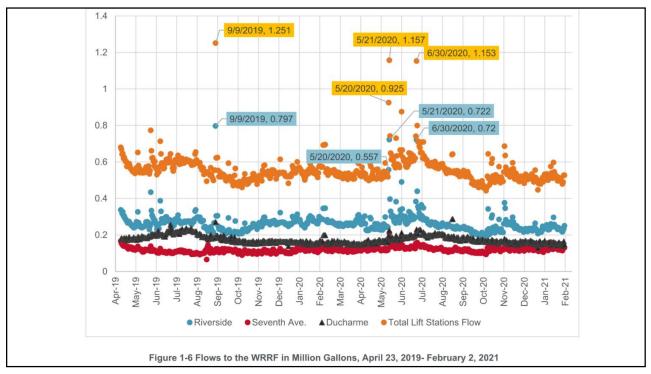


2022 Water System Improvements – No Grants	
Category	Cost
Total Project Cost	\$ 3,829,000
Treasure State Endowment Program Grant (TSEP)	\$ O
ARPA Grant	\$ O
USDA Rural Development Grant	\$ O
Total Grants	\$ O
Total Cost to be Financed	\$ 3,829,000
Interest Rate	3.75%
Number of Years	20
Coverage	20%
Monthly Debt Service including Coverage	\$ 27, 242
Monthly Increase in O&M	\$ 0
Number of EDU's	3,179
Monthly Rate Per EDU	\$ 8.56

2022 Water System Improvements – ARPA Grants	
Category	Cost
Total Project Cost	\$ 3,829,000
Treasure State Endowment Program Grant (TSEP)	\$ 0
ARPA Grant	\$ 2,375,353
USDA Rural Development Grant	\$ 0
Total Grants	\$ O
Total Cost to be Financed	\$ 908,647
Interest Rate	3.75%
Number of Years	20
Coverage	20%
Monthly Debt Service including Coverage	\$ 6,464
Monthly Increase in O&M	\$ O
Number of EDU's	3,179
Monthly Rate Per EDU	\$ 2.03

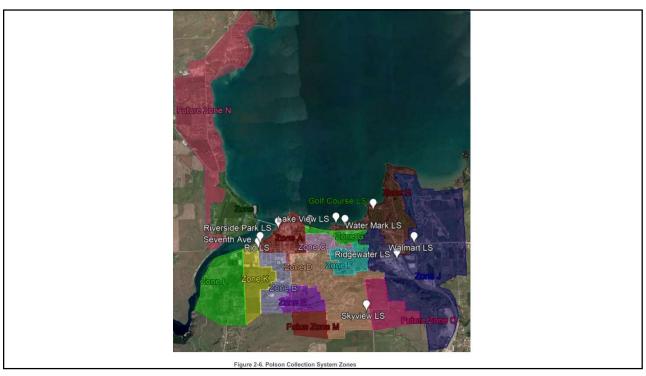


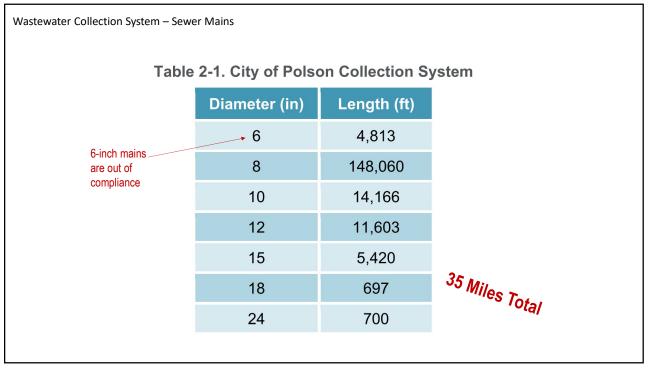
Т	able 1-10 Polson Gallon	s Per Capita Per Day (GP	CD)				
Timeframe	Total Volume, Day of Accumulation	Population Estimate for 2019 ¹	Gallons Per Capita Per Day (GPCD)				
April 23, 2019 – December 31, 2019	141.4 MG, 253 days						
January 1, 2020 – December 31, 2020	202.9 MG, 365 days ²						
January 1, 2021, February 2, 2021	17.1 MG, 33 days						
Total	361.4 MG, 651 days						
Average Day Flow	555,105 GPD 385 GPM	5060	109.7				
 Notes: ¹ The population for 2019 is the estimate from Census data. The years 2020 were not available at this time of this report. ² While 2020 was a leap year, the WRRF Plant SCADA skipped February 17th, 2020 for an unknown reason therefore there are 365 days of data. It should be noted that the gallons per capita per day represents the total metered water use (including commercial, industrial, and irrigation), divided by the total population. Given the wide variation in customer type and usage per zone, GPCD should not be used as a basis for actual per customer wastewater flows. 							



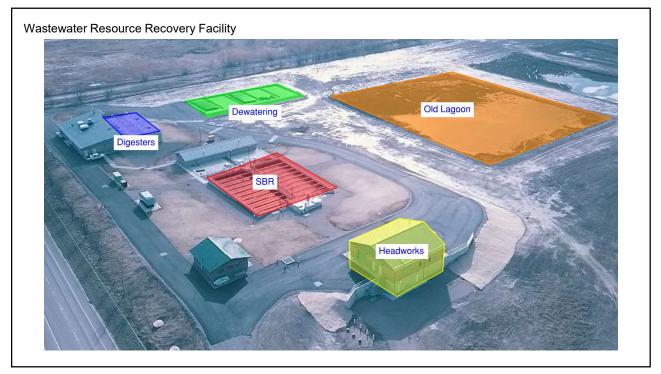
Projected Wastewater Demands			
Table 1-11 Projected Pop	ulation and Wastew	vater Flows for 204	1
Growth Rates	0.65% per year	1.37% per year	2.28% per year
Population	5835	6826	8309
Peaking Factor	3.18	3.12	3.03
Average Day Volume (Million Gallons)	0.642	0.751	0.914
Average Flow (Gallons Per Minute)	446	521	635
Maximum Day Volume (Million Gallons)	1.01	1.19	1.44
Maximum Flow (Gallons Per Minute)	704	824	1003
Peak Hourly Demand (Gallons per Minute)	1417	1627	1923
Average Year (Million Gallons)	234	274	337

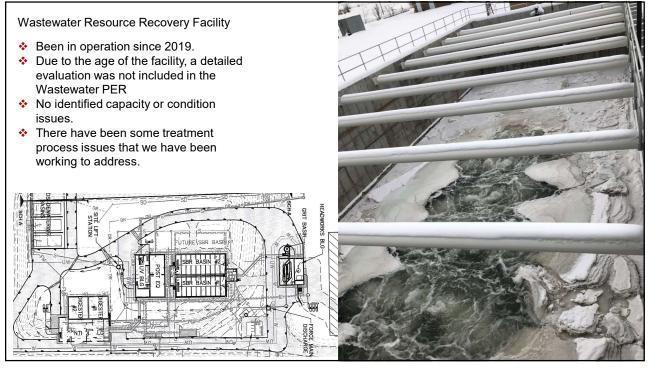
For planning, modeling and design purposes, the data associated with the 1.37% annual population growth will be used moving forward.





Vastewater Collection Sy			Station Inst	allation/Upgrade Year	
Lift Station	Installation Date	Upgrade Date	No. of Pumps	Pump Manufacturer	Capacity (gpm)
Golf Course	2003		2	Gorman Rupp	719
T.P. Mark	Unknown		2	GR-JSV3B60-X2.7	100
Lake View Village	1975	2013	2	Flygt 3102, N type impellers	290
West Shore	1996		3	Unknown	265
Riverside Park	1993	2013	3	Cornell	863
DuCharme Park	1994	2006	2	71 HP G.R. Big Grinders	1117
Rio	1989	2000	2	Flygt 3092, N type impellers	150
Seventh Avenue	1999		2	Gorman Rupp	640
Skyview	2010		2	Gorman Rupp	250
Tundra	2008		2	Gormunn Rupp and "Samurai Pump"	215
Walmart	2012		2	GR-SFV4C / Japan	150





Condition Assessr	nent – Collection System					
	Table 2-15. Co	llection System C	ondition	Assessment		
Collection System Zone	Sewer Line Location	Project Name	Dia. (in)	Condition Assessment	мос	Condition Notes
Zone A	Between 5th Ave W / 6th Ave W and Between Main St / 1st St E	Bank Alley	8	Failing	Clay Tile	Gravel, cannot camera
Zone A	5th Ave W, between 4th St W and 2nd St W	Riverside Addition Lines	8	Failing	Clay Tile	Separated clay tile
Zone A	Between 4th Ave W / 5th Ave W and 2nd St W and 1st St W	Riverside Addition Lines	8	Failing	Clay Tile	Separated clay tile
Zone C	Between 3rd Ave E / 4th Ave E and Between 5th St E / 7th St E	Subway Line	10	Failing	Clay Tile	Grease, shallow slope
Zone C	Between 4th Ave E / 5th Ave E and Between 5th St E / Immaculate Conception Catholic Church	Church Line	10	Failing	Clay Tile	Aggregate exposed, drop
Zone D	Between 11th Ave E / 12th Ave E and 1st St E / 2nd St E	City Shop Alley	8	Failing	Clay Tile	Several backups
Zone D	Between 12th Ave E / 15th Ave E and5th St E / 6th St E	Brown's Line	8	Failing	Clay Tile	Roots, backup
Zone H	Country Club Dr.	Golf to Lakeview Line	8	Failing	Clay Tile	

Condition Assessment – Lift Stations

Table 2-16: Lift Station Condition Assessment

Lift Station	Comments
Golf Course Lift Station	New backup generator
T.P. Mark Lift Station	New backup generator
Lake View Village Lift Station	Complete replacement
West Shore Lift Station	New backup generator and move the control panel above ground
Riverside Park Lift Station	Remodel and update, install second wet well to increase wet well capacity, and eliminate overflow to the Flathead River
All Lift Stations	Tie all lift stations into SCADA with remote monitoring at WWTP

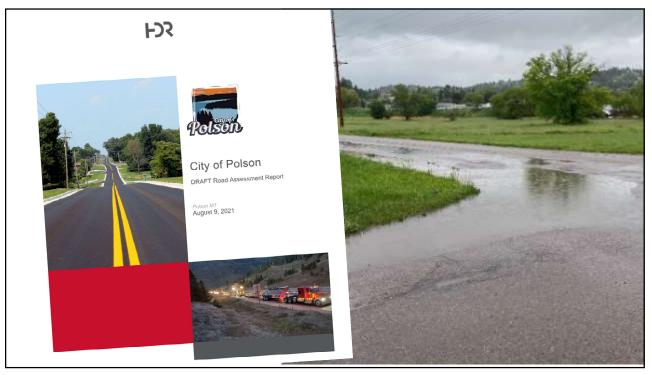




Recommen	ded Improveme	ents – Near Term							
				Table 7-1. 202 Recommended		Recommended	Cost Estimate	Funding Sources	Schedule
Project Description Lift Station: Riverside Park	Location Riverside Park LS	Need Remodel and update, install second wet well to increase wet well capacity, and eliminate overflow to the Flathead River	Priority High	Timeline Near term - 1 to 3 years	Consequence if not Corrected Backup of raw sewage into basements and overflow into Flathead Lake 1.2 & 3	Alternative • Update and remodel LS	(2020) \$1,200,000	ARPA Minimum Allocation Grant	September 2021 through December
Lift Station: Lake View	Lake View LS	Complete Replacement	High	Near term - 1 to 3 years	Backup of raw sewage into basements and overflow into Flathead Lake 1, 2 & 3	Replace LS	\$500,000	ARPA Local Fiscal Recovery Funds ARPA Competitive	2022
Collection System: Hillcrest	Line to run along Hillcrest to Seventh LS	New line to alleviate system capacity issues	High	Near term - 1 to 3 years	Backup of raw sewage into basements and overflow into Flathead Lake 1, 2 & 3	Replace clay tile pipes	\$200,000	Grant	
Lift Station: All	All Lift Stations	Tie all lift stations into SCADA with remote monitoring at WWTP	High	Near term - 1 to 3 years	Failure to have network control of entire collection system resulting in backup of raw sewage into basements and overflow into Flathead Lake 1, 2 & 3	Upgrade SCADA	\$250,000		

2022 Wastewater System Improvements – Potential Rate Determin No Grants	ation
Category	Cost
Total Project Cost	\$ 2,150,000
Treasure State Endowment Program Grant (TSEP)	\$ O
ARPA Grant	\$ O
USDA Rural Development Grant	\$ 0
Total Grants	\$ O
Total Cost to be Financed	\$ 2,150,000
Interest Rate	3.75%
Number of Years	20
Coverage	20%
Monthly Debt Service including Coverage	\$ 15,296
Monthly Increase in O&M	\$ O
Number of EDU's	3,179
Monthly Rate Per EDU	\$ 4.81

2022 Wastewater System Improvements – Potential Rate Determina ARPA Grant	ation
Category	Cost
Total Project Cost	\$ 2,150,000
Treasure State Endowment Program Grant (TSEP)	\$ O
ARPA Grant	\$ 2,150,000
USDA Rural Development Grant	\$ O
Total Grants	\$ O
Total Cost to be Financed	\$ O
Interest Rate	3.75%
Number of Years	20
Coverage	20%
Monthly Debt Service including Coverage	\$ O
Monthly Increase in O&M	\$ O
Number of EDU's	3,179
Monthly Rate Per EDU	\$ 0.00



Roadway Classifications

- Residential roadways are characterized as having low speeds and traffic volumes while serving as local streets that provide direct access to adjacent properties (Road Design Manual, MDT 2016). An example of a residential roadway is 15th Ave. E. as it serves to connect private residents to the 7th St. E. collector.
- Collectors are characterized as systems that move less volumes of traffic than arterials and serve as intermediate links between the arterials and points of origin (Road Design Manual, MDT 2016). The City's 12th Ave. E. is an example of a collector roadway as it connects residential and local roads to the two primary arterials, 1st. St. E. and 2nd St. W.
- Arterials are generally characterized as road systems that move and provide travel between major points while also serving adjacent properties. These systems play an important role in inter- and intra-urban circulation (A Policy on Geometric Design of Highways and Streets: AASHTO 2018).



Table 1. Severity Characteristics					
RATING	PAVEMENT CONDITION	SUBGRADE PERFORMANCE	DRAINAGE PERFORMANCE	ADA COMPLIANCE	
1	Minor cracking and pavement deterioration	No evidence of settlement	No evident ponding	Pedestrian ramps, crossings, and walkways meet ADA/PROWAG requirements	
2	Moderate cracking and minor potholing	Minor alligator cracking and settlement evident	Minor ponding and lack of adequate drainage features	Pedestrian ramps, crossings, and walkways do not meet ADA/PROWAG requirements	
3	Major cracking, potholing, and rutting. Evident failure of the pavement surface	Major settlement with depressed road section.	Major ponding and no drainage features provided	Pedestrian facilities missing where required	



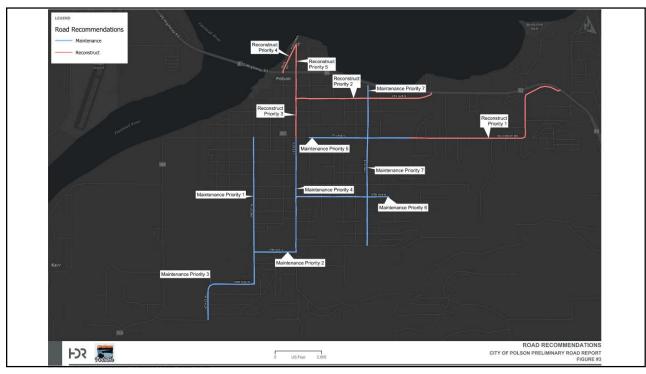












Road Maintenance Recommendations							
Table 2. Preliminary Roadway Maintenance Recommendations							
Maintenance Recommendations							
Priority	Street Name	Estimated Work Type	Estimated Cost	Comments			
1	2nd St W	Crack Seal, Chip Seal	\$200k - \$310k	\$1.11/sf - \$1.73/sf			
2	17th Ave W/E	Overlay	\$190k - \$300k	\$5.68/sf – \$8.97/sf			
3	19th Ave W	Chip Seal	\$60k - \$95k	\$0.67/sf - \$1.06/sf			
4	1st St E – S of 7 th	Mill/Fill	\$550k - \$865k	Pending Geotech analysis			
5	7th Ave E	Overlay	\$485k - \$760k	\$5.11/sf - \$8.01/sf			
6	12th Ave E	Overlay	\$330k - \$515k	\$5.00/sf - \$7.80/sf			
7	7th St E	Chip Seal W/ Some Overlay	\$380k - \$590k	\$3.14/sf - \$4.88/sf			

Road Reconstruction Recommendations

Table 3. Preliminary Roadway Reconstruction Recommendations.

Reconstruction Recommendations						
Priority	Street Name	Estimated Work Type	Estimate Cost	Comments		
1	Hillcrest Dr	Reconstruction & Widening	\$1.55M - \$2.35M	Address SUP crossing area, Drainage		
2	4 Ave E	Full Reconstruction	\$3.00M - \$4.50M	C&G, Sidewalk, ADA		
3	1st St E - South of 93	Shoulder Recon. & Mill/Fill	\$1.50M - \$2.25M	C&G failed, subgrade failure		
4	Main St E - North of 93	Partial Reconstruction	\$750K – \$1.15M	Optional Mill/fill		
5	1 St E - North of 93	Partial Reconstruction	\$650K – \$950k	Optional Mill/Fill		



Table 1-4. Historical US Bureau of Census Data

Census Date	Population	Annual Average Growth Rate
1960	2,314	-
1970	2,464	0.65%
1980	2,798	1.36%
1990	3,291	1.76%
2000	4,041	2.28%
2010	4,488	1.11%
2019 ¹	5,060	1.42%

¹2020 Census results are expected to be available in Spring of 2021, the 2019 population is an estimate.